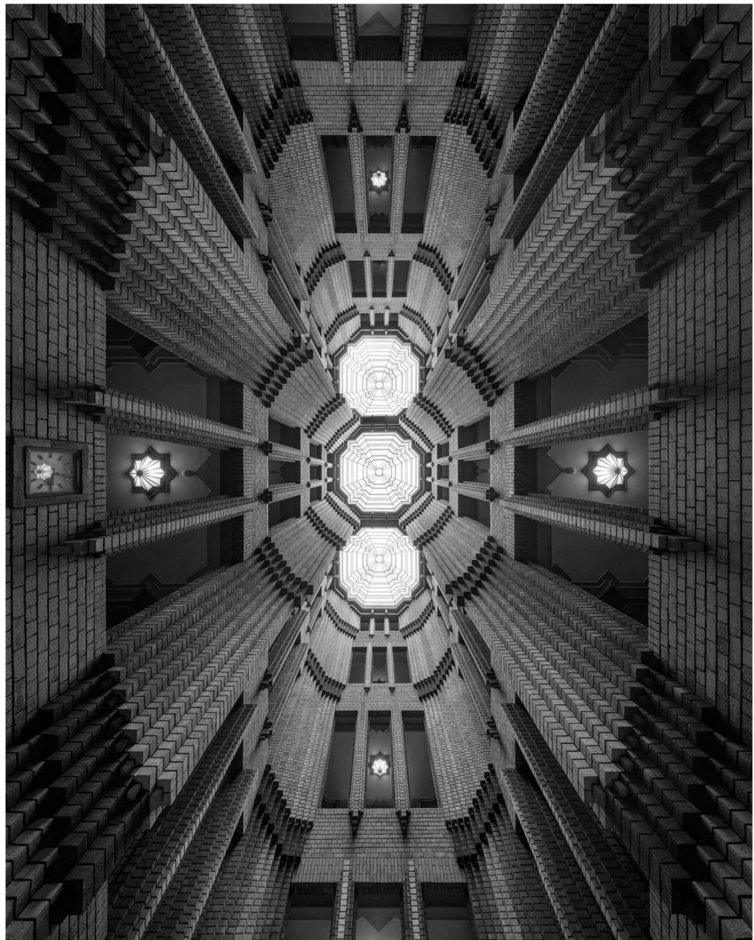


Strength Design Unreinforced

- TMS Chap 9 – Strength Design

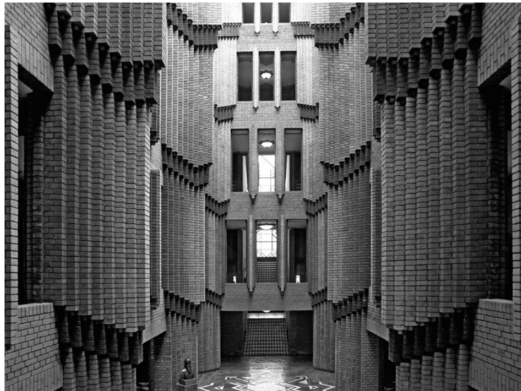
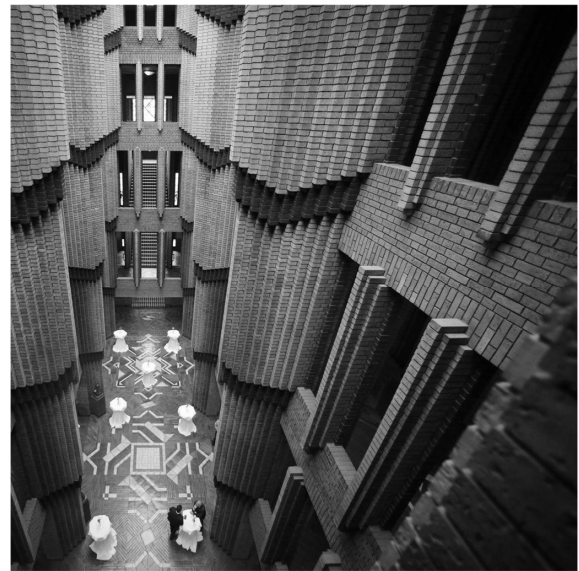
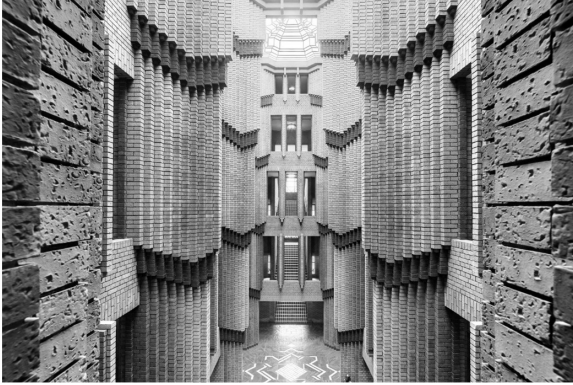
Hoechst Fabrik
Technical Administration Bldg.
Peter Behrens, 1924



Hoechst Fabrik Technical Admin. Bldg. Peter Behrens, 1924



Hoechst Fabrik
Technical Admin. Bldg.
Peter Behrens, 1924

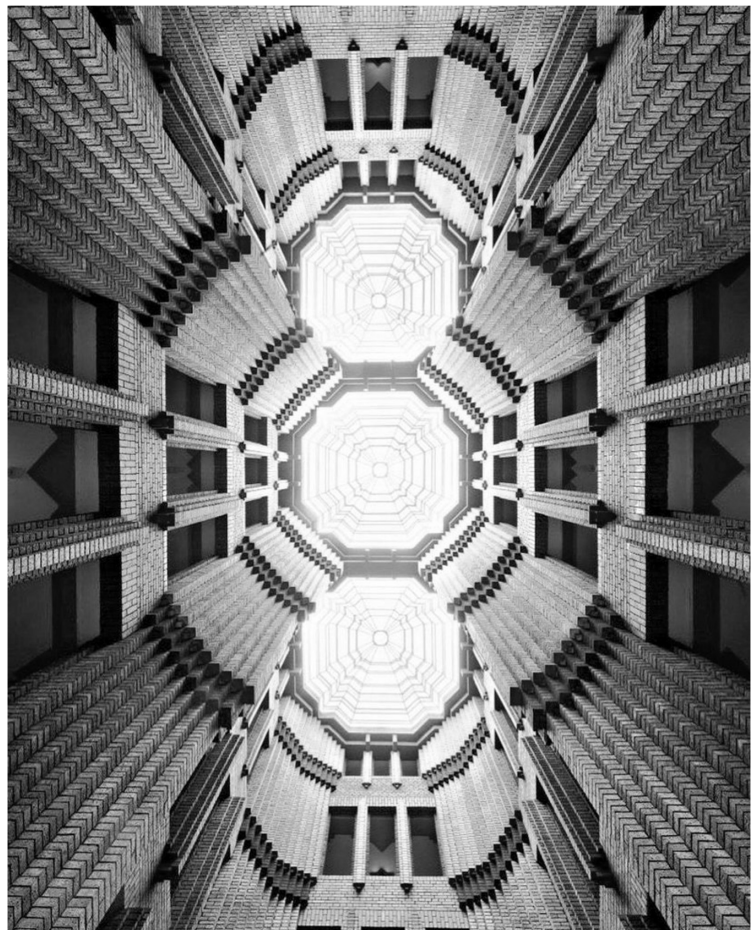


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Design Options

Empirical

- TMS 402 Appendix A

Allowable Stress Design (ASD)

- TMS 402 Chap. 8

Strength Design

- TMS 402 Chap. 9

TMS 402/602-16

Building Code Requirements and Specification for Masonry Structures

Containing

TMS 402-16 *Building Code Requirements for Masonry Structures*
(Formerly also designated as ACI 530 and ASCE 5)

TMS 602-16 *Specification for Masonry Structures*
(Formerly also designated as ACI 530.1 and ASCE 6)

and Companion Commentaries

Advancing the knowledge of masonry



Design Options

Empirical

- TMS 402 Appendix A

Allowable Stress Design (ASD)

- TMS 402 Chap. 8

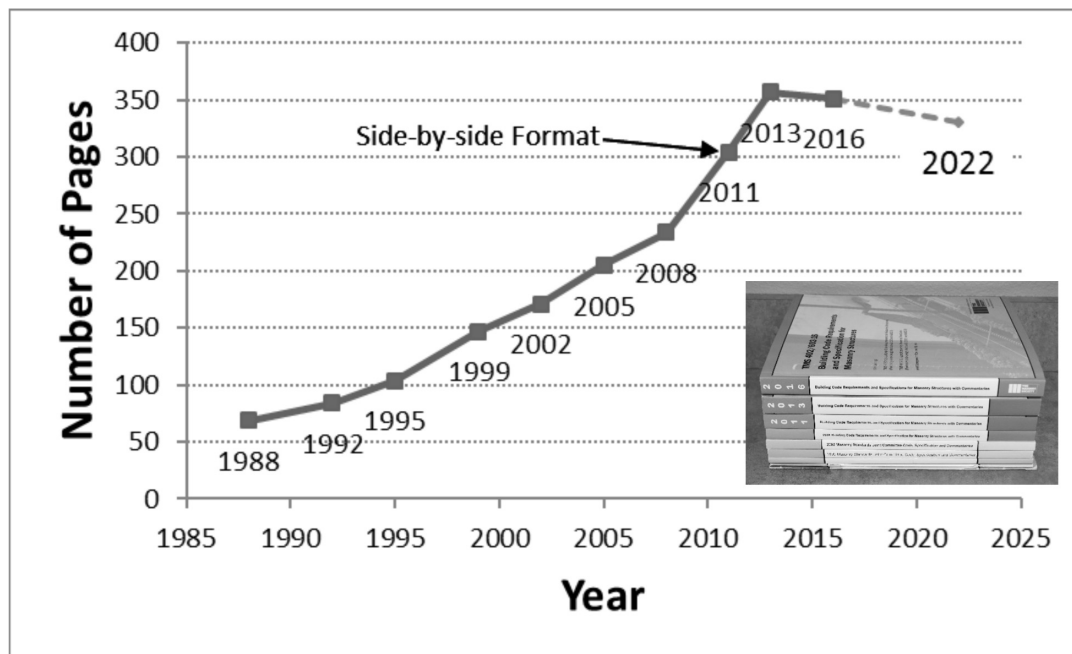
Strength Design

- TMS 402 Chap. 9
- IBC Section 2108
 - mostly references TMS 402
 - steel development length not less than 12" or more than $72 d_b$
 - Mechanical and welded splices



TMS 402

Development



TMS 402 Chapter 9

9.1 General

9.1.1 Scope

9.1.2 Required strength

9.1.3 Design strength

9.1.4 Strength-reduction factors

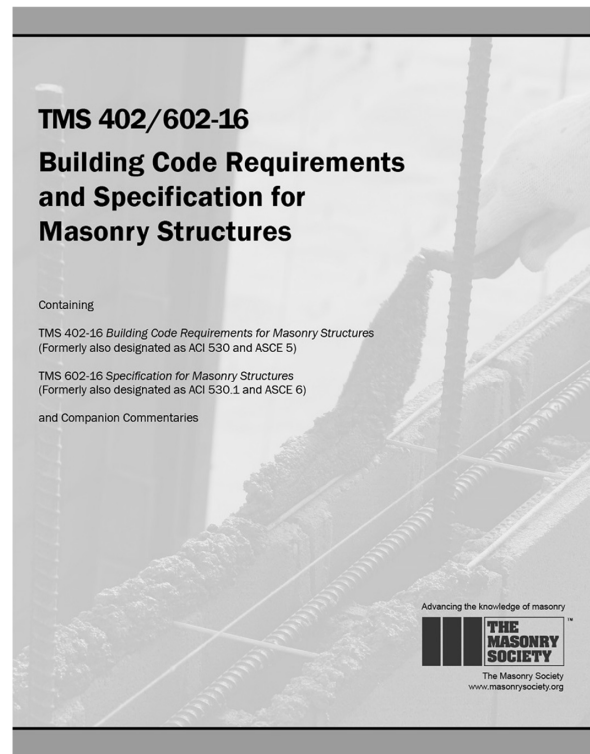
9.1.5 Deformation requirements

9.1.6 Anchor bolts embedded in grout

9.1.7 Shear strength in Multiwythe elements

9.1.8 Nominal bearing strength

9.1.9 Material properties



TMS 402 Chapter 9

Strength Design Method

Strength Required by Loads < Strength of Masonry

$$\gamma P_u < \phi P_n$$

- 1.4 D
- 1.2 D + 1.6 Lr + 0.5(Lr or S)
- 1.2 D + 1.6(Lr or S) + (L or 0.5W)
- 1.2 D + 1.0W + L + 0.5(Lr or S)
- 0.9D + 1.0W
- 1.2D + Ev + Eh + L + 0.2S
- 0.9D - Ev + Eh

(Equation 9-11) for $h/r < 99$

$$P_n = 0.80 \left\{ 0.80 A_n f'_m \left[1 - \left(\frac{h}{140r} \right)^2 \right] \right\}$$

(Equation 9-12) for $h/r > 99$

$$P_n = 0.80 \left[0.80 A_n f'_m \left(\frac{70r}{h} \right)^2 \right]$$

TMS 402 Chapter 9

9.1.4 Strength-reduction factors, ϕ

Action	Reinforced Masonry	Unreinforced Masonry
combinations of flexure and axial load	0.90	0.60
shear	0.80	
bearing	0.60	
anchor bolts: pryout	0.50	
anchor bolts: controlled by anchor bolt steel	0.90	
anchor bolts: pullout	0.65	

TMS 402 Chapter 9 Strength Design $P_u \leq P_n$

Section 9.2 Unreinforced (plain) masonry

9.2.1 Scope

9.2.2 Design criteria (remain uncracked)

9.2.3 Design assumptions

- strain proportional to distance from N.A.
- flexural tension proportional to strain
- flexural comp. + axial comp. proportional to strain
- stresses in reinforcement are not accounted for in resistance to loads

$$f_t = \frac{Mc}{I} - \frac{P}{A}$$

9.2.4 Nominal flexural and axial strength for unreinforced masonry in axial + flexure

- compressive stress $\leq 0.80 f'_m$
- tensile stress \leq modulus of rupture, f_r

(Equation 9-11) for $h/r < 99$

$$P_n = 0.80 \left\{ 0.80 A_n f'_m \left[1 - \left(\frac{h}{140r} \right)^2 \right] \right\}$$

9.2.5 Axial tension

- tension resistance shall be neglected

(Equation 9-12) for $h/r > 99$

$$P_n = 0.80 \left[0.80 A_n f'_m \left(\frac{70r}{h} \right)^2 \right]$$

9.2.6 Nominal shear strength lesser of $(3.8 A_{nv} \sqrt{f'_m})$ or $300 A_{nv}$

TMS 402 Chapter 9

Table 9.1.9.2 Modulus of Rupture

Masonry Type	Mortar Type			
	Portland cement/lime or mortar cement		Masonry Cement	
	M or S	N	M or S	N
Normal to Bed Joints				
Solid Units	133	100	80	51
Hollow Units ¹				
UngROUTed	84	64	51	31
Fully Grouted	163	158	153	145
Parallel to bed joints in running bond				
Solid Units	267	200	160	100
Hollow Units				
UngROUTed and partially grouted	167	127	100	64
Fully grouted	267	200	160	100
Parallel to bed joints not laid in running bond				
Continuous grout section parallel to bed joints	335	335	335	335
Other	0	0	0	0

Masonry Analysis – strength design procedure for axial compression using TMS 402 (2016) Strength Design (LRFD) – **non-reinforced**

Given: applied load, geometry, material

Find: axial compressive load capacity, P_n

- Determine the masonry strength, f'_m , based on unit strength, f_u , and mortar type
- Find the net area, A_n , and r (see TEK 14-1B)
- Calculate h/r
- Choose the axial strength equation, P_n :
If $h/r < 99$ use TMS 402 eq.9-11
If $h/r > 99$ use TMS 402 eq.9-12
- Calculate ϕP_n where ϕ for axial force = 0.60
- Check that ϕP_n is greater than P_u .

(Equation 9-11) for $h/r < 99$

$$P_n = 0.80 \left\{ 0.80 A_n f'_m \left[1 - \left(\frac{h}{140r} \right)^2 \right] \right\}$$

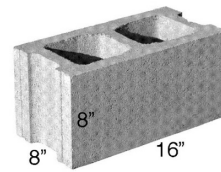
(Equation 9-12) for $h/r > 99$

$$P_n = 0.80 \left[0.80 A_n f'_m \left(\frac{70r}{h} \right)^2 \right]$$

Masonry Strength

TMS 602 – Table 2 – s18

Masonry strength, f_m , based on unit strength, f_u , and mortar type M, S or N



Concrete Masonry

Table 2 — Compressive strength of masonry based on the compressive strength of concrete masonry units and type of mortar used in construction

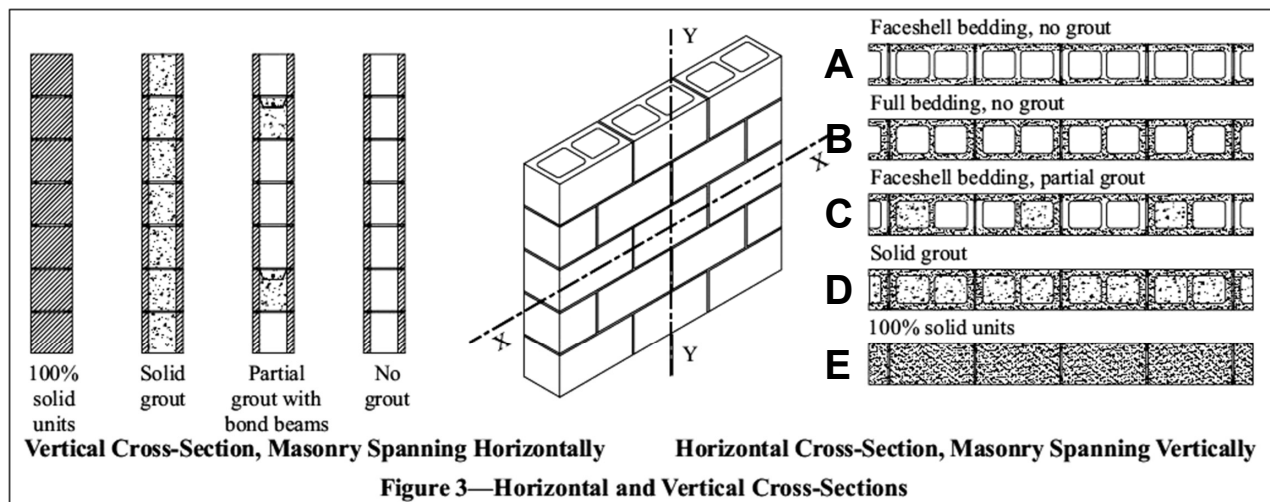
Net area compressive strength of concrete masonry, psi (MPa) ¹	Net area compressive strength of ASTM C90 concrete masonry units, psi (MPa)	
	Type M or S mortar	Type N mortar
1,750 (12.07)	---	2,000 (13.79)
2,000 (13.79)	2,000 (13.79)	2,650 (18.27)
2,250 (15.51)	2,600 (17.93)	3,400 (23.44)
2,500 (17.24)	3,250 (22.41)	4,350 (28.96)
2,750 (18.96)	3,900 (26.89)	----
3,000 (20.69)	4,500 (31.03)	----

¹ For units of less than 4 in. (102 mm) nominal height, use 85 percent of the values listed.

Masonry Analysis

for axial compression using TMS 402 (2016)
Strength Design – **non-reinforced**

Section Properties of Concrete Masonry Walls TEK 14 – 1B



Reinforced Masonry Analysis

for axial compression using TMS 402 (2016)
Strength Design – **non-reinforced**

Section Properties of Concrete Masonry Walls TEK 14 – 1B

Table 3—8-inch (203-mm) Single Wythe Walls, 1¼ in. (32 mm) Face Shells (standard)

3a: Horizontal Section Properties (Masonry Spanning Vertically)					
Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties ^A		
			A_n (in. ² /ft)	I_n (in. ⁴ /ft)	S_n (in. ³ /ft)
A Hollow	No grout	Face shell	30.0	308.7	81.0
B Hollow	No grout	Full	41.5	334.0	87.6
D/E 00% solid/solidly grouted		Full	91.5	443.3	116.3
C Hollow	16	Face shell	62.0	378.6	99.3
↓ Hollow	24	Face shell	51.3	355.3	93.2
↓ Hollow	32	Face shell	46.0	343.7	90.1
↓ Hollow	40	Face shell	42.8	336.7	88.3
↓ Hollow	48	Face shell	40.7	332.0	87.1
↓ Hollow	72	Face shell	37.1	324.3	85.0
↓ Hollow	96	Face shell	35.3	320.4	84.0
↓ Hollow	120	Face shell	34.3	318.0	83.4

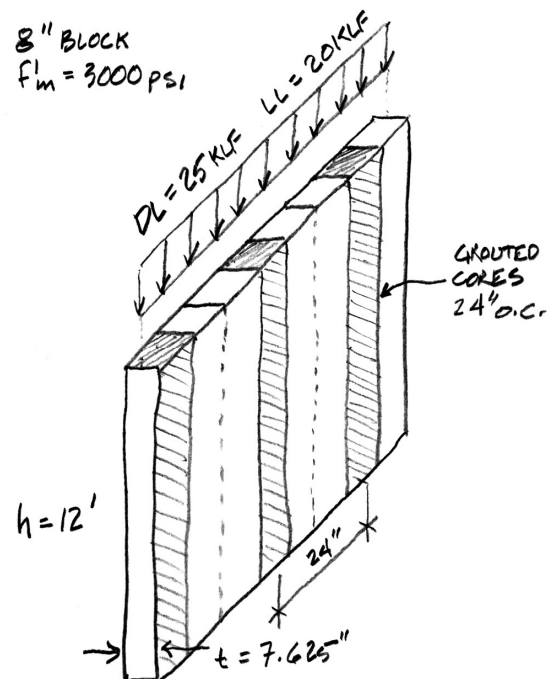
Reinforced Masonry Analysis

for axial compression using TMS 402 (2016)
Strength Design – **non-reinforced**

Example Problem

Given: geometry: 8" block, grouted 24" o.c.
material: $f'_m = 3000$ psi
Find: check pass/fail for the given loading

- Determine the masonry strength, f'_m , based on unit strength, f_u , and mortar type. (given $f'_m = 3000$ psi)



Faceshell bedding, partial grout



Reinforced Masonry Analysis

for axial compression using TMS 402 (2016)
Strength Design – **non-reinforced**

2. Find the net area, A_n , and radius of gyration, r_{avg} (see TEK 14-1B)

3a: Horizontal Section Properties (Masonry Spanning Vertically)									
Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties ^A			Average cross-sectional properties ^B			
			A_n (in. ² /ft)	I_n (in. ⁴ /ft)	S_n (in. ³ /ft)	A_{avg} (in. ² /ft)	I_{avg} (in. ⁴ /ft)	S_{avg} (in. ³ /ft)	r_{avg} (in.)
Hollow	No grout	Face shell	30.0	308.7	81.0	41.5	334.0	87.6	2.84
Hollow	No grout	Full	41.5	334.0	87.6	41.5	334.0	87.6	2.84
100% solid/solidly grouted		Full	91.5	443.3	116.3	91.5	443.3	116.3	2.20
Hollow	16	Face shell	62.0	378.6	99.3	65.8	387.1	101.5	2.43
Hollow	24	Face shell	51.3	355.3	93.2	57.7	369.4	96.9	2.53
Hollow	32	Face shell	46.0	343.7	90.1	53.7	360.5	94.6	2.59
Hollow	40	Face shell	42.8	336.7	88.3	51.2	355.2	93.2	2.63
Hollow	48	Face shell	40.7	332.0	87.1	49.6	351.7	92.2	2.66
Hollow	72	Face shell	37.1	324.3	85.0	46.9	345.8	90.7	2.71
Hollow	96	Face shell	35.3	320.4	84.0	45.6	342.8	89.9	2.74
Hollow	120	Face shell	34.3	318.0	83.4	44.8	341.0	89.5	2.76

3b: Vertical Section Properties (Masonry Spanning Horizontally)									
Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties ^A			Average cross-sectional properties ^B			
			A_n (in. ² /ft)	I_n (in. ⁴ /ft)	S_n (in. ³ /ft)	A_{avg} (in. ² /ft)	I_{avg} (in. ⁴ /ft)	S_{avg} (in. ³ /ft)	r_{avg} (in.)
Hollow	No grout	Face shell	30.0	308.7	81.0	40.5	330.1	86.6	2.86
Hollow	No grout	Full	30.0	308.7	81.0	41.5	334.0	87.6	2.84
100% solid/solidly grouted		Full	91.5	443.3	116.3	91.5	443.3	116.3	2.20
Hollow	16	Face shell	60.8	376.0	98.6	71.2	397.4	104.2	2.36
Hollow	24	Face shell	50.5	353.6	92.7	61.0	374.9	98.3	2.48
Hollow	32	Face shell	45.4	342.4	89.8	55.8	363.7	95.4	2.55
Hollow	40	Face shell	42.3	335.6	88.0	52.8	357.0	93.6	2.60
Hollow	48	Face shell	40.3	331.1	86.9	50.7	352.5	92.5	2.64
Hollow	96	Face shell	35.1	319.9	83.9	45.6	341.3	89.5	2.74
Hollow	120	Face shell	34.1	317.7	83.3	44.6	339.0	88.9	2.76

Table 3-8-inch (203-mm) Single Wythe Walls, 1½ in. (32 mm) Face Shells (standard)

Reinforced Masonry Analysis

for axial compression using TMS 402
(2016) Strength Design – **non-reinforced**

3. Calculate h/r

FROM TEK 14-01B FOR 8" CMU SINGLE WYTHE HOLLOW BLOCK GROUT @ 24" O.C. FACE SHELL MORTAR

$$A_n = 51.3 \text{ in}^2$$

$$r_{avg} = 2.53 \text{ in}$$

$$\frac{h}{r} = \frac{144}{2.53} = 56.92 < 99 \therefore \text{EQ 9-11}$$

4. Choose the axial strength equation, P_n :

If $h/r < 99$ use TMS 402 eq.9-11

If $h/r > 99$ use TMS 402 eq.9-12

(Equation 9-11) for $h/r < 99$

$$P_n = 0.80 \left\{ 0.80 A_n f'_m \left[1 - \left(\frac{h}{140r} \right)^2 \right] \right\}$$

Reinforced Masonry Analysis

for axial compression using TMS 402

(2016) Strength Design – **non-reinforced**

(Equation 9-11) for $h/r < 99$

$$P_n = 0.80 \left\{ 0.80 A_n f'_m \left[1 - \left(\frac{h}{140r} \right)^2 \right] \right\}$$

5. Calculate ϕP_n
 where ϕ for axial force = 0.60
 (unreinforced)

$$P_n = 0.8 \left[0.8 A_n f'_m \left(1 - \frac{h}{140r} \right)^2 \right]$$

$$P_n = 0.8 \left[0.8 (51.3 \frac{\text{in}^2}{\text{ft}}) (3 \text{ ksi}) \left(1 - \left(\frac{144''}{140(2.53'')} \right)^2 \right) \right]$$

$$P_n = 0.8 \left[123.12 (0.835) \right] = 0.8 [102.77]$$

$$P_n = 0.8 [102.77] = 82.2 \text{ K/ft}$$

6. Check that ϕP_n is greater than P_u .

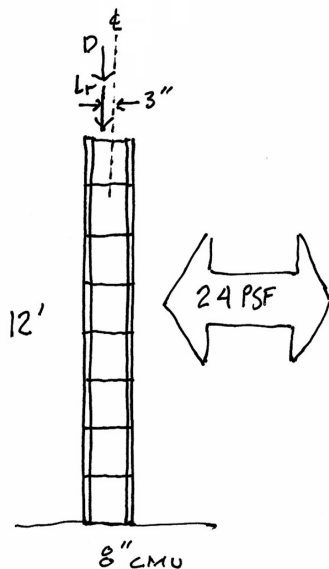
$$\phi P_n = 0.6 (82.2 \text{ KLF}) = 49.33 \text{ KLF}$$

$$P_u = 1.2 (25 \text{ KLF}) + 1.6 (20 \text{ KLF}) = 62 \text{ KLF}$$

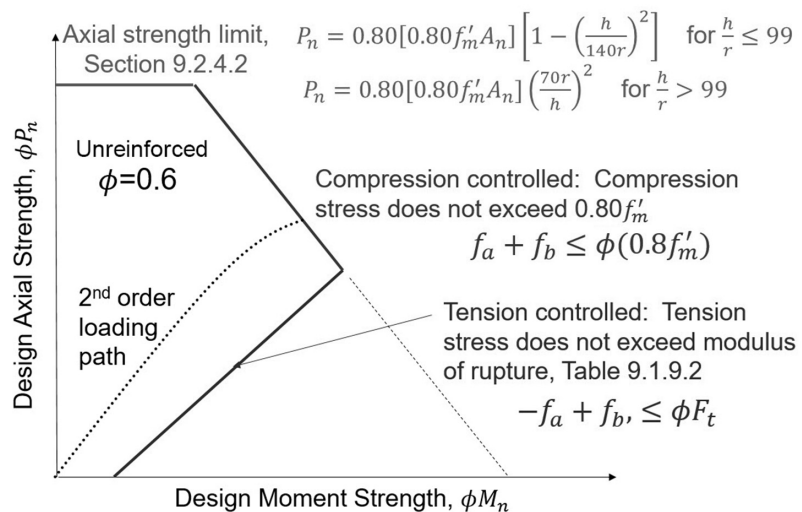
$$P_u = 62 \text{ KLF} > 49.3 \text{ KLF} = \phi P_n \therefore \text{NG!}$$

TMS 402 Chapter 9

Axial Load + Flexure



Interaction Diagram



TMS 402 Chapter 9

Moment Magnification

Design for magnified moment: $M_u = \psi M_{u,0}$

$$\psi = \frac{1}{1 - \frac{P_u}{P_e}} = \frac{1}{1 - \frac{P_u}{A_n f'_m \left(\frac{70r}{h}\right)^2}}$$

- Can take $\psi = 1$ if $h/r \leq 45$
- Can take $\psi = 1$ if $45 < h/r \leq 60$ and nominal strength (P_n) reduced by 10%

Unreinforced Masonry Wall

example

Given:

$h = 12$ ft

$t = 8$ in hollow CMU, $f_u = 2000$ psi

type S mortar, face shell bedding, no grout

Loading:

$D = 1$ k/ft + selfweight of 30 psf

$L_r = 0.5$ k/ft ($e = 3$ in.)

$W = 24$ psf (+ or -)

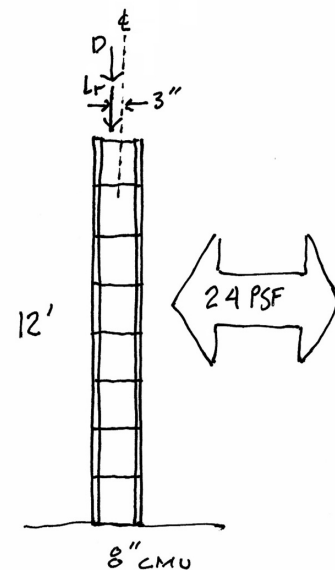
Load Combinations

1.4D

1.2D + 1.6 L_r

1.2D + 1.0W + 0.5 L_r

0.9D + 1.0W



Unreinforced Masonry Wall

example

Given:

$f_u = 2000$ psi

type S mortar

$f'_m = 2000$ psi (TMS 602 – table 2 – s18)

Table 2 — Compressive strength of masonry based on the compressive strength of concrete masonry units and type of mortar used in construction

Net area compressive strength of concrete masonry, psi (MPa) ¹	Net area compressive strength of ASTM C90 concrete masonry units, psi (MPa)	
	Type M or S mortar	Type N mortar
1,750 (12.07)	---	2,000 (13.79)
2,000 (13.79)	2,000 (13.79)	2,650 (18.27)
2,250 (15.51)	2,600 (17.93)	3,400 (23.44)
2,500 (17.24)	3,250 (22.41)	4,350 (28.96)
2,750 (18.96)	3,900 (26.89)	----
3,000 (20.69)	4,500 (31.03)	----

¹ For units of less than 4 in. (102 mm) nominal height, use 85 percent of the values listed.

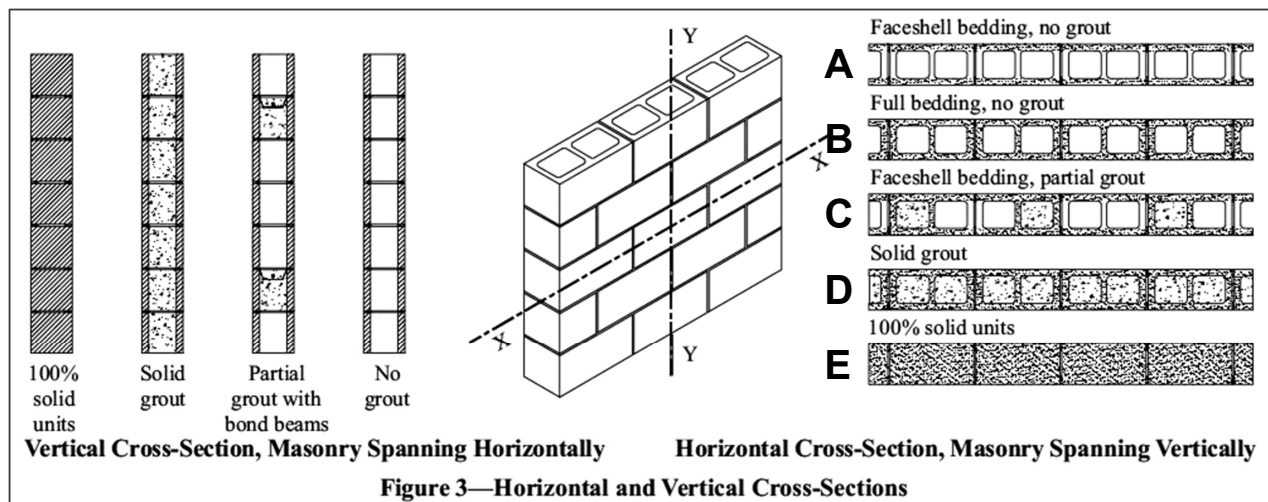
Unreinforced Masonry Wall

example

Section Properties of Concrete Masonry Walls

TEK 14 – 1B

using A – faceshell bedding, no grout



Unreinforced Masonry Wall

example

$$A_n = 30 \text{ in}^2/\text{ft}$$

$$S_n = 81 \text{ in}^3/\text{ft}$$

$$r_{\text{avg}} = 2.84 \text{ in}$$

$$\frac{h}{r} = \frac{144 \text{ in.}}{2.84 \text{ in.}} = 50.7$$

(TEK 14 – 1B)

3a: Horizontal Section Properties (Masonry Spanning Vertically)									
Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties ^A			Average cross-sectional properties ^B			
			A_n (in. ² /ft)	I_n (in. ⁴ /ft)	S_n (in. ³ /ft)	A_{avg} (in. ² /ft)	I_{avg} (in. ⁴ /ft)	S_{avg} (in. ³ /ft)	r_{avg} (in.)
Hollow	No grout	Face shell	30.0	308.7	81.0	41.5	334.0	87.6	2.84
Hollow	No grout	Full	41.5	334.0	87.6	41.5	334.0	87.6	2.84
100% solid/solidly grouted		Full	91.5	443.3	116.3	91.5	443.3	116.3	2.20
Hollow	16	Face shell	62.0	378.6	99.3	65.8	387.1	101.5	2.43
Hollow	24	Face shell	51.3	355.3	93.2	57.7	369.4	96.9	2.53
Hollow	32	Face shell	46.0	343.7	90.1	53.7	360.5	94.6	2.59
Hollow	40	Face shell	42.8	336.7	88.3	51.2	355.2	93.2	2.63
Hollow	48	Face shell	40.7	332.0	87.1	49.6	351.7	92.2	2.66
Hollow	72	Face shell	37.1	324.3	85.0	46.9	345.8	90.7	2.71
Hollow	96	Face shell	35.3	320.4	84.0	45.6	342.8	89.9	2.74
Hollow	120	Face shell	34.3	318.0	83.4	44.8	341.0	89.5	2.76

3b: Vertical Section Properties (Masonry Spanning Horizontally)									
Unit	Grout spacing (in.)	Mortar bedding	Net cross-sectional properties ^A			Average cross-sectional properties ^B			
			A_n (in. ² /ft)	I_n (in. ⁴ /ft)	S_n (in. ³ /ft)	A_{avg} (in. ² /ft)	I_{avg} (in. ⁴ /ft)	S_{avg} (in. ³ /ft)	r_{avg} (in.)
Hollow	No grout	Face shell	30.0	308.7	81.0	40.5	330.1	86.6	2.86
Hollow	No grout	Full	30.0	308.7	81.0	41.5	334.0	87.6	2.84
100% solid/solidly grouted		Full	91.5	443.3	116.3	91.5	443.3	116.3	2.20
Hollow	16	Face shell	60.8	376.0	98.6	71.2	397.4	104.2	2.36
Hollow	24	Face shell	50.5	353.6	92.7	61.0	374.9	98.3	2.48
Hollow	32	Face shell	45.4	342.4	89.8	55.8	363.7	95.4	2.55
Hollow	40	Face shell	42.3	335.6	88.0	52.8	357.0	93.6	2.60
Hollow	48	Face shell	40.3	331.1	86.9	50.7	352.5	92.5	2.64
Hollow	96	Face shell	35.1	319.9	83.9	45.6	341.3	89.5	2.74
Hollow	120	Face shell	34.1	317.7	83.3	44.6	339.0	88.9	2.76

Table 3-8-inch (203-mm) Single Wythe Walls, 1½ in. (32 mm) Face Shells (standard)

Unreinforced Masonry Wall example

Load Combinations

1.4D

1.2D + 1.6L_r

1.2D + 1.0W + 0.5L_r

0.9D + 1.0W

Determine the controlling load combination

- A. 1.2D + 1.0W + 0.5L_r Wind pressure (compression on outside)
- B. 1.2D + 1.0W + 0.5L_r Wind suction (compression on inside)
- C. 0.9D + 1.0W Wind pressure (compression on outside)
- D. 0.9D + 1.0W Wind suction (compression on inside)

Load Combination	Tensile Stress (psi)
1.2D + 1.0W + 0.5L _r wind pressure	?
1.2D + 1.0W + 0.5L _r wind suction	?
0.9D + 1.0W wind pressure	?
0.9D + 1.0W wind suction	?

Unreinforced Masonry Wall example

Load Combinations

1.4D

1.2D + 1.6L_r

1.2D + 1.0W + 0.5L_r

0.9D + 1.0W

Determine the controlling load combination

- A. 1.2D + 1.0W + 0.5L_r Wind pressure (compression on outside)
- B. 1.2D + 1.0W + 0.5L_r Wind suction (compression on inside)
- C. 0.9D + 1.0W Wind pressure (compression on outside)
- D. 0.9D + 1.0W Wind suction (compression on inside)

Load Combination	Tensile Stress (psi)
1.2D + 1.0W + 0.5L _r wind pressure	5.9 psi
1.2D + 1.0W + 0.5L _r wind suction	40.4 psi
0.9D + 1.0W wind pressure	12.4 psi
0.9D + 1.0W wind suction	47.6 psi

see slide 34 for calculation

Unreinforced Masonry Wall example

Given:

h = 12 ft

t = 8 in hollow CMU, f_u = 2000 psi

type S mortar, face shell bedding, no grout

Loading:

D = 1 k/ft + selfweight of 30 psf

L_r = 0.5 k/ft (w/ e=3 in.)

W = 24 psf (+ or -)

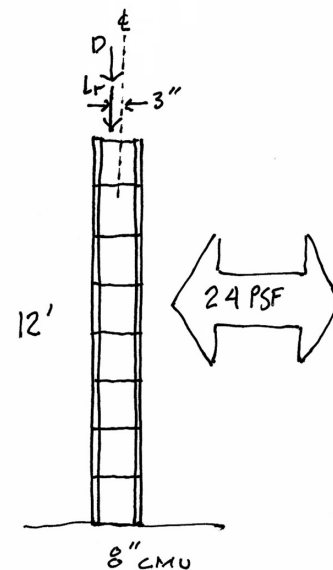
Load Combinations

1.4D

1.2D + 1.6L_r

1.2D + 1.0W + 0.5L_r

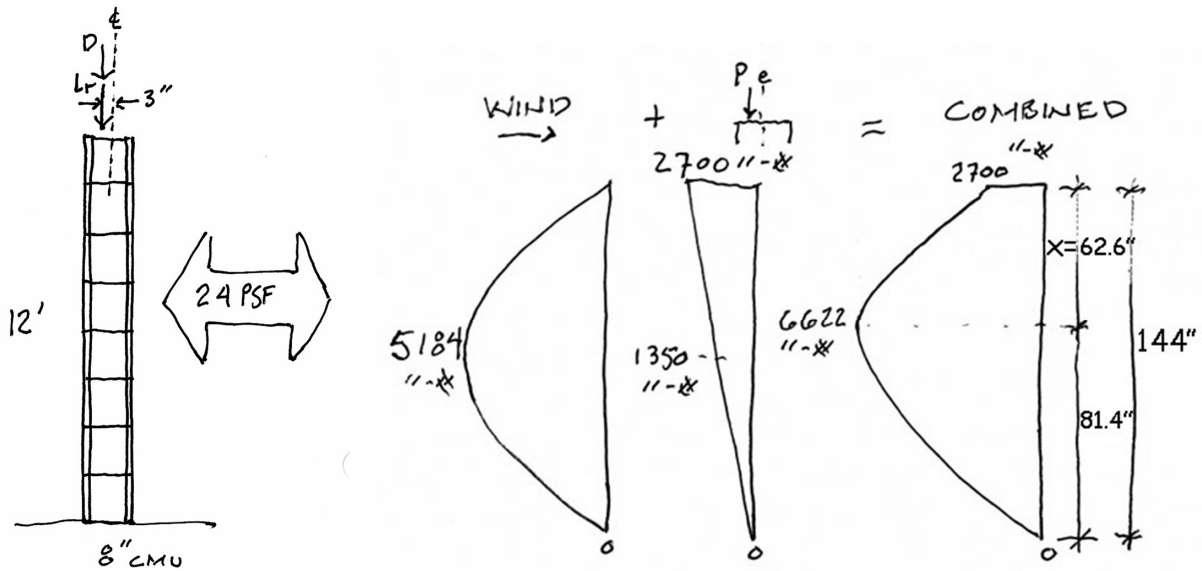
0.9D + 1.0W



Unreinforced Masonry Wall

example

0.9D + 1.0W Wind suction (compression on inside)



Unreinforced Masonry Wall example

0.9D + 1.0W - Wind suction (compression on inside face of wall)

Moment at top of wall from floor, M_{uf}

$$M_{uf} = P_{uf}e = 0.9 \left(1000 \frac{\text{lb}}{\text{ft}} \right) (3.0 \text{ in.}) = 2700 \frac{\text{lb} \cdot \text{in.}}{\text{ft}}$$

Location of max moment, x

$$x = \frac{h}{2} - \frac{M_{uf}}{w_u h} = \frac{144 \text{ in.}}{2} - \frac{2700 \frac{\text{lb} \cdot \text{in.}}{\text{ft}}}{24 \frac{\text{lb}}{\text{ft}^2} (12 \text{ ft})} = 62.6 \text{ in.}$$

Factored axial load, P_u

$$P_u = 0.9 \left(1000 \frac{\text{lb}}{\text{ft}} + 30 \frac{\text{lb}}{\text{ft}^2} \left(\frac{62.6 \text{ in.}}{12 \text{ in./ft}} \right) \right) = 1041 \frac{\text{lb}}{\text{ft}}$$

Maximum combined moment, $M_{u,0}$

$$\begin{aligned} M_{u,0} &= \frac{M_{uf}}{2} + \frac{w_u h^2}{8} + \frac{M_{uf}^2}{2w_u h^2} \\ &= \frac{2700 \frac{\text{lb} \cdot \text{in.}}{\text{ft}}}{2} + \frac{24 \frac{\text{lb}}{\text{ft}^2} (12 \text{ ft})^2 \left(12 \frac{\text{in.}}{\text{ft}} \right)}{8} + \frac{\left(2700 \frac{\text{lb} \cdot \text{in.}}{\text{ft}} \right)^2}{2 \left(24 \frac{\text{lb}}{\text{ft}^2} \right) (12 \text{ ft})^2 \left(12 \frac{\text{in.}}{\text{ft}} \right)} \\ &= 1350 + 5184 + 88 = 6622 \frac{\text{lb} \cdot \text{in.}}{\text{ft}} \end{aligned}$$

Unreinforced Masonry Wall example

0.9D + 1.0W - Wind suction (compression on inside face of wall)

9.2.4.3 P-Delta effects

9.2.4.3.1 Members shall be designed for the strength level axial load, P_u , and the moment magnified for the effects of member curvature, M_u .

9.2.4.3.2 The magnified moment, M_u , shall be determined either by a second-order analysis, or by a first-order analysis and Equations 9-13 and 9-14.

$$M_u = \psi M_{u,0} \quad (\text{Equation 9-13})$$

$$\psi = \frac{1}{1 - \frac{P_u}{A_n f'_m \left(\frac{70r}{h}\right)^2}} \quad (\text{Equation 9-14})$$

9.2.4.3.3 A value of $\psi = 1$ shall be permitted for members in which $h/r \leq 45$.

Moment Magnifier:
$$\psi = \frac{1}{1 - \frac{P_u}{A_n f'_m \left(\frac{70r}{h}\right)^2}} = \frac{1}{1 - \frac{1041 \frac{lb}{ft}}{\left(30 \frac{in.^2}{ft}\right) (2000 psi) \left(\frac{70 \cdot 2.84}{144}\right)^2}} = 1.01$$

Unreinforced Masonry Wall example

0.9D + 1.0W - Wind suction (compression on inside face of wall)

Moment Magnifier:
$$\psi = 1.01$$

Compression Stress:
$$\frac{P_u}{A_n} + \frac{\psi M_{u,0}}{S_n} = \frac{1041 \frac{lb}{ft}}{30 \frac{in.^2}{ft}} + \frac{1.01 \left(6622 \frac{lb \cdot in.}{ft}\right)}{81.0 \frac{in.^3}{ft}} = 34.7 + 82.5 = 117.4 \text{ psi}$$

(f'm)
 Nominal Strength = 0.8(2000 psi) = 1600 psi
 Design Strength = 0.6(1600 psi) = 960 psi **OK**

Tension stress:
$$+\frac{P_u}{A_n} - \frac{\psi M_{u,0}}{S_n} = +34.7 - 82.5 = 47.6 \text{ psi tension}$$

Nominal Strength = 51 psi (f_r) Design Strength = 0.6 (51 psi) = 30.6 psi **NG!**

Unreinforced Masonry Wall example

0.9D + 1.0W - Wind suction (compression on inside face of wall)

What to do to make wall work? (short of reinforcing wall)

1. Increase wall size, say to 12 in. ($S_n = 139.6 \text{ in}^3/\text{ft}$). Maximum tensile stress is 12.7 psi.
2. Grout wall. ($w\text{-wall} = 75 \text{ psf}$; $A_n = 91.5 \text{ in}^2/\text{ft}$; $S_n = 116.3 \text{ in}^3/\text{ft}$; $f_r = 163 \text{ psi}$) Maximum tensile stress of 45.3 psi is less than design stress of $0.6(163) = 97.8 \text{ psi}$
3. Use Portland cement/lime or mortar cement (modulus of rupture is 84 psi). Maximum tensile stress of 47.2 psi is less than design stress of $0.6(84)=50.4 \text{ psi}$
4. Use pilasters